## GENERAL NOTES

Note A: For type of block and joint finish, see other sheets.

Note B: When blocks are laid in stacked bond, ladder type, galvanized joint reinforcement shall be provided. A minimum of 2-3.76 mm wires continuous at 1219 maximum to be used. Locate reinforcement in joints

that are at the approximate midpoint between bond beams. Note C: Horizontal joints shall be tooled concave or may

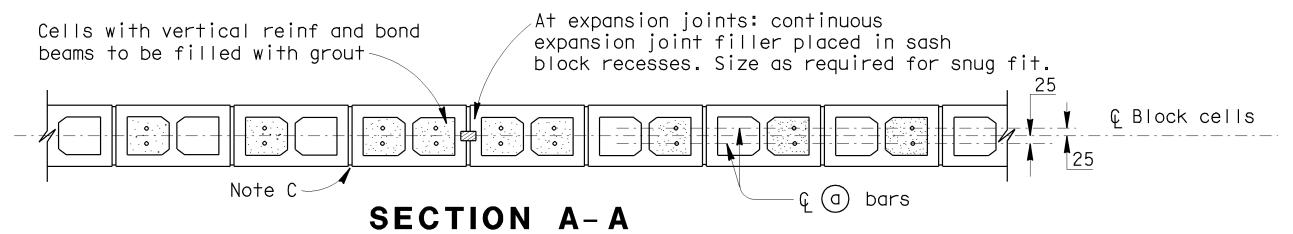
Vertical joints shall be tooled concave or may be raked.

Note D: For intermediate wall heights that are between the "H's" given, use the tabular information for the next higher "H".

Note E: Masonry strengths are listed in the "SOUNDWALL REINFORCEMENT TABLE".

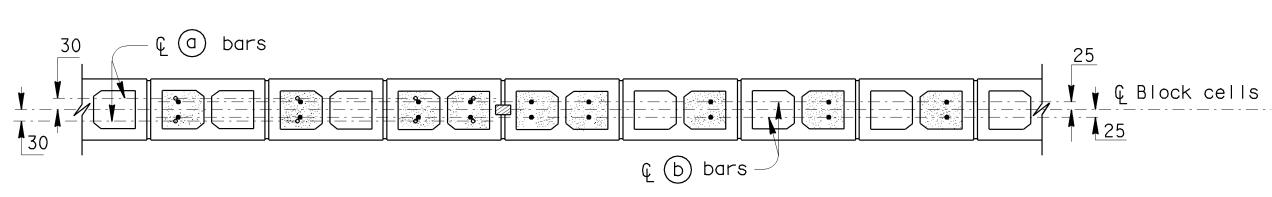
be raked.

Note F: Geotechnical Services should confirm the stability of the slopes.



For details not shown, see other sections

H = 1829THRU H=3048



SECTION A-A

SECTION B-B

For details not shown, see other sections

H=3658 THRU H=4877



DIST.	COUNTY	ROUTE	KILOMETER POST TOTAL PROJECT	SHEET NO.	TOTAL SHEETS

REGISTERED ENGINEER - CIVIL

PLANS APPROVAL DATE The State of California or its officers or agent

shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.

**DESIGN NOTES** 

Uniform Building Code, 1997 Edition and the Bridge Design Specifications.

DESIGN WIND LOAD

REINFORCED CONCRETE

f'c = 22.41 MPa

fy = 413.7 MPa

DESIGN

DESIGN SEISMIC LOAD

958 Pa for all heights (H)

0.57 Dead load

**CONCRETE MASONRY** 

n = 19.3

REGULAR STRENGTH

f'm = 10.34 MPa

fm = 3.41 MPa

n = 25.8

fs = 165.5 MPa

f'm = 13.79 MPa f'm = 17.24 MPafh = 4.55 MPa  $f_b = 5.72 \text{ MPa}$ fs = 165.5 MPa fs = 165.5 MPa

HIGH STRENGTH

n = 15.5

LOAD FACTORS AND LOAD COMBINATIONS

Working Stress Design ( WSD ) Percentage of unit stress 100%

Group 1: D + E + SC Group 2: D + W + SC + E 100% Group 3: D + 0.71 EQD + E 100%

Load Factor Design ( LFD ) Where:

D = Dead load Group A: BD +1.7 E + 1.7 SC E = Lateral earth pressure Group B: BD +1.7 E + 1.3 W Group C: BD +1.3 E + 1.0 EQE SC = Live load surcharge Group D: BD + 1.3 E + 1.0 EQD W = Wind load Group E: BD + 1.1 E + 0.85 ( EQE + EQD )

EQD = Seismic dead load Where: B = 0.9 or 1.2, whichever controls in design

D = Dead load E = Lateral earth pressure

SC = Live load surcharge W = Wind load EQD = Seismic dead load

EQE = Seismic earth load

Concrete masonry:

STRENGTH REDUCTION FACTORS, Ø

Reinforced concrete:

For flexure \_\_\_\_\_ Ø = 0.80

For flexure \_\_\_\_\_  $\emptyset = 0.90$ For shear\_\_\_\_\_ Ø = 0.85

Note 1: Concrete masonry designed by the Strength Design Method. Pile caps and piles designed for flexure and shear ( Internal stability ) by the Strength Design Method.

Note 2: Case 1 - Level ground on both sides of the soundwall: Pile embedment lengths (External stability) were determined using the Sheet Pile Procedure with Service Loads and a Factor of Safety for overturning of 2.0. Allowable net lateral soil pressures (Q) of  $38.2 \text{ kN/m}^2/\text{m}$  (  $24\overline{3} \text{ psf/ft}$  ),  $62.1 \text{ kN/m}^2/\text{m}$ ( 395 psf/ft ) and 101 kN/m /m ( 643 psf/ft ) were considered based on a Log Spiral Analysis using 67% of the ultimate passive pressure. These values correspond with angles of shearing resistance  $(\emptyset)$  of 25°, 30° and 35° respectively and unit weights of soil ( $\nearrow$ ) of 17.27 kN/m³ (110 pcf), 18.05 kN/m³ ( 115 pcf ) and 18.84 kN/m³ ( 120 pcf ) respectively. An Isolation Factor of 3.0 was used on both sides.

Note 3: Case 2 - Level ground on one side of the soundwall and sloping ground on the opposite side: Pile embedment lengths (External stability) were determined using the Sheet Pile Procedure with Service Loads and a factor of Safety for overturning of 2.0. Allowable net lateral soil pressures for the sloping side of the wall (Q) of 10.5 kN/m<sup>2</sup>/m (67 psf/ft), and 21.68 kN/m<sup>2</sup>/m ( 134 psf/ft ) were considered based on a Log Spiral Analysis using 67% of the ultimate passive pressure. These values correspond with angles of shearing resistance (Ø) of 30° and 35° respectively and unit weights of soil ( $\nearrow$ ), 18.05 kN/m<sup>3</sup> ( 115 pcf ) and 18.84 kN/m³ ( 120 pcf ) respectively. Isolation factors were 3.0 for the level side and 2.0 for the sloping side.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE SHOWN

		NO SCALE	PLOT1
STANDARD DRAWING	STATE OF	BRIDGE NO.	COUNDWALL MACONDY BLOCK ON DUE CAD
RELEASE 9/8/03 DESIGN BY D. DUNRUD CHECKED W.C. WALKER	CALIFORNIA	DIVISION OF	SOUNDWALL - MASONRY BLOCK ON PILE CAP
DETAILS BY RYFF CHECKED W.C. WALKER	CALIFORNIA DEPARTMENT OF TRANSPORTATION	ENGINEERING SERVICES KILOMETER POST	DETAILS NO. 2
SUBMITTED BY D. DUNRUD  DRAWING 11/94  OFFICE CHIEF	DEPARTMENT OF TRANSPORTATION		DETAILS NO. 2
DS OSD 2147A (METRIC) (REV. 2/25/97)	ORIGINAL SCALE IN MILLIMETERS	DISREGARD PRI	REVISION DATES (PRELIMINARY STAGE ONLY)  SHEET OF  ON DATES

USERNAME => jsanchez

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